

Assessment of Geotechnical Parameter Uncertainty Using Probabilistic Methods in Slope Stability Analysis

William Araujo¹, Tomas Garces², Gaby Ruiz¹

Universidad de Piura

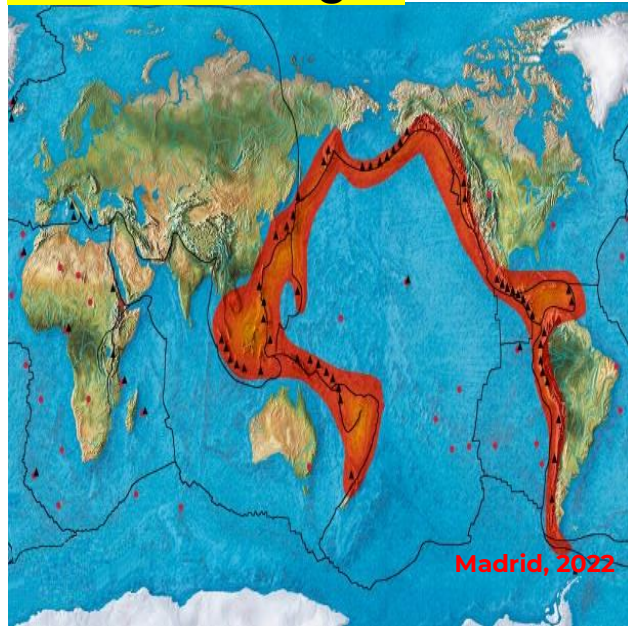
**CITC-14 | SEPTEMBER 2-5, 2024
HOSTED BY FEDERAL UNIVERSITY OF RIO DE JANEIRO
RIO DE JANEIRO, BRAZIL**

CITC GLOBAL
Construction in the 21st Century



Introduction & Background

1. Geodynamics and climate change :



2. Infrastructure



3. Risk management



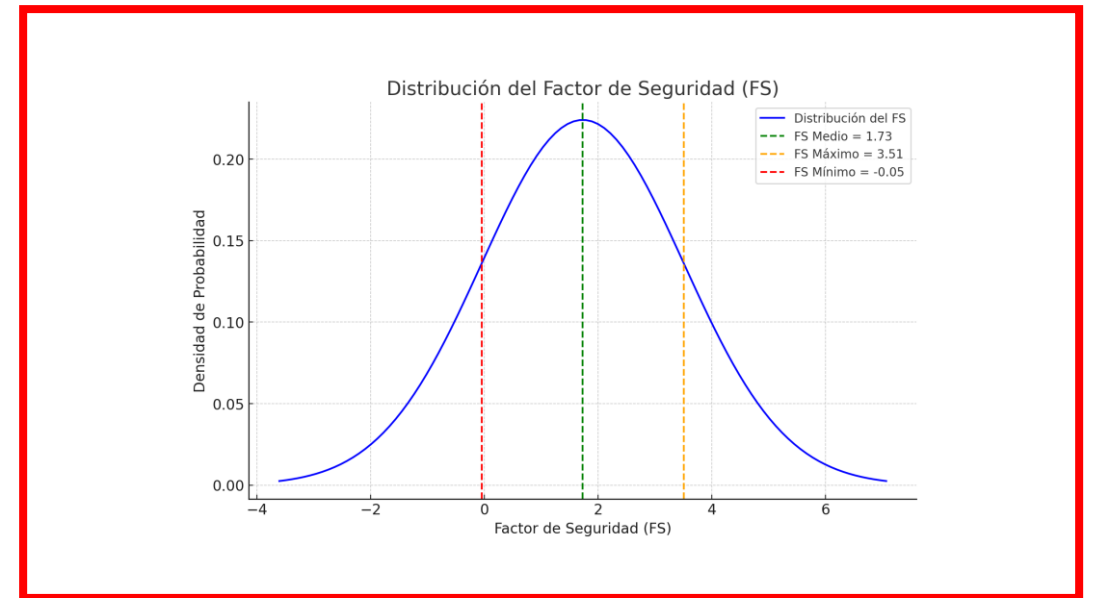
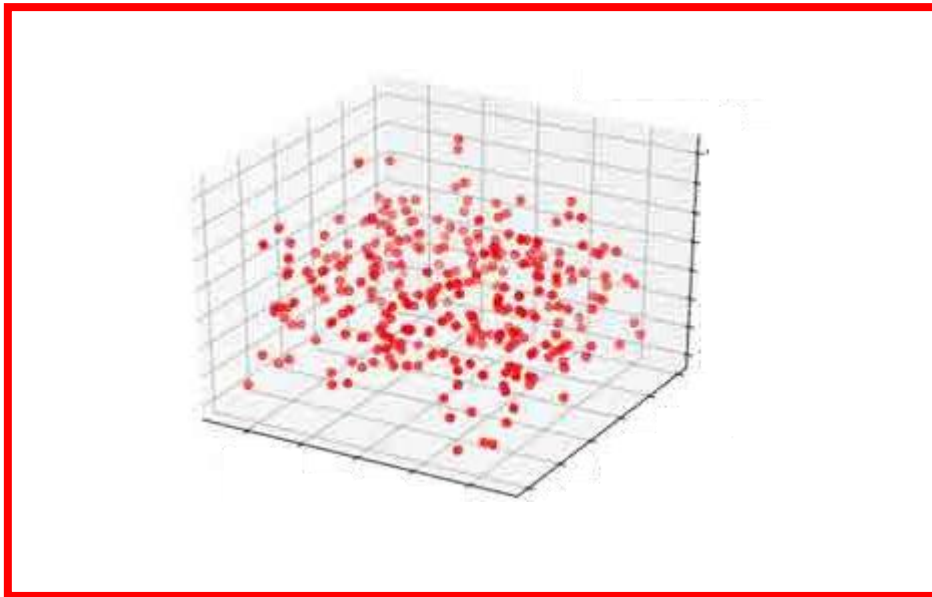
More than 400 destructive events
per year (Economist, 2017)

Aim, Objectives, and Scope

- Evaluate the uncertainty of geotechnical parameters using probabilistic methods in slope stability analysis,
- To define the geometry and characterize the geotechnical properties and boundary conditions of the synthetic slope models proposed for stability analyses based on a reference case from the literature.
- Determine the reliability and probability of failure of synthetic slopes by applying the probabilistic methods of Latin Hypercube and First Order - Second Moment, and compare them with deterministic evaluation methods.

Research Design and Methodology

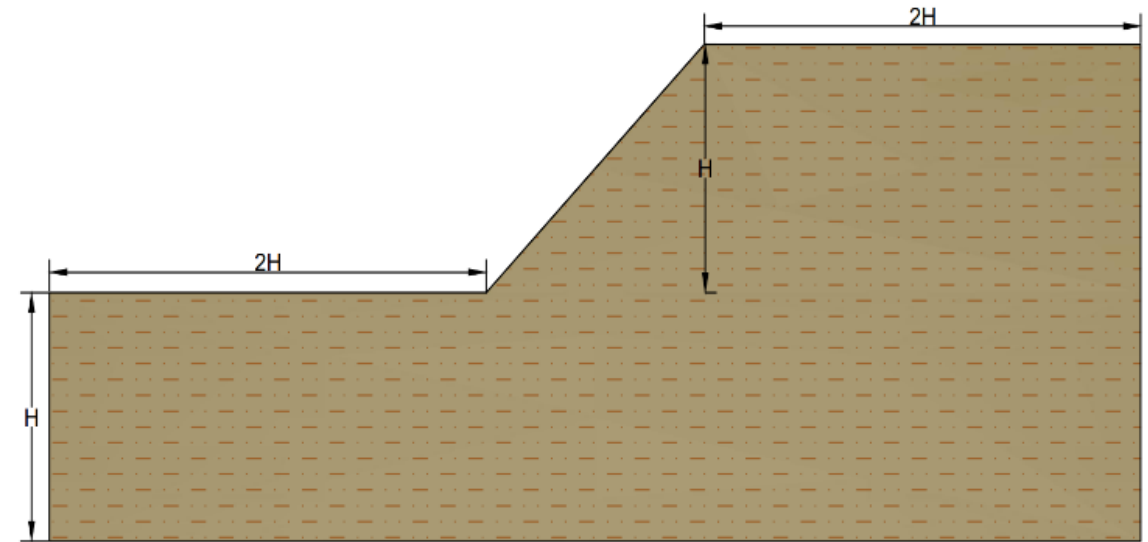
1. Latin Hypercube Sampling (LHS)
2. First order second moment method (FOSM)



Research Design and Methodology

Table 2. Summary of data for this study

	Parameters	Units	Values
Geometrics	Height (h)	m	10, 20, 30
	Slope (H:V)	-	1, 1.5, 2
	Cohesion (c)	kN/m ²	18 (CV=20.0%)
Geotechnical	Specific weight (γ)	kN/m ³	18 (CV=3.00%)
	Friction angle (ϕ)	°	30 (CV=3.70%)



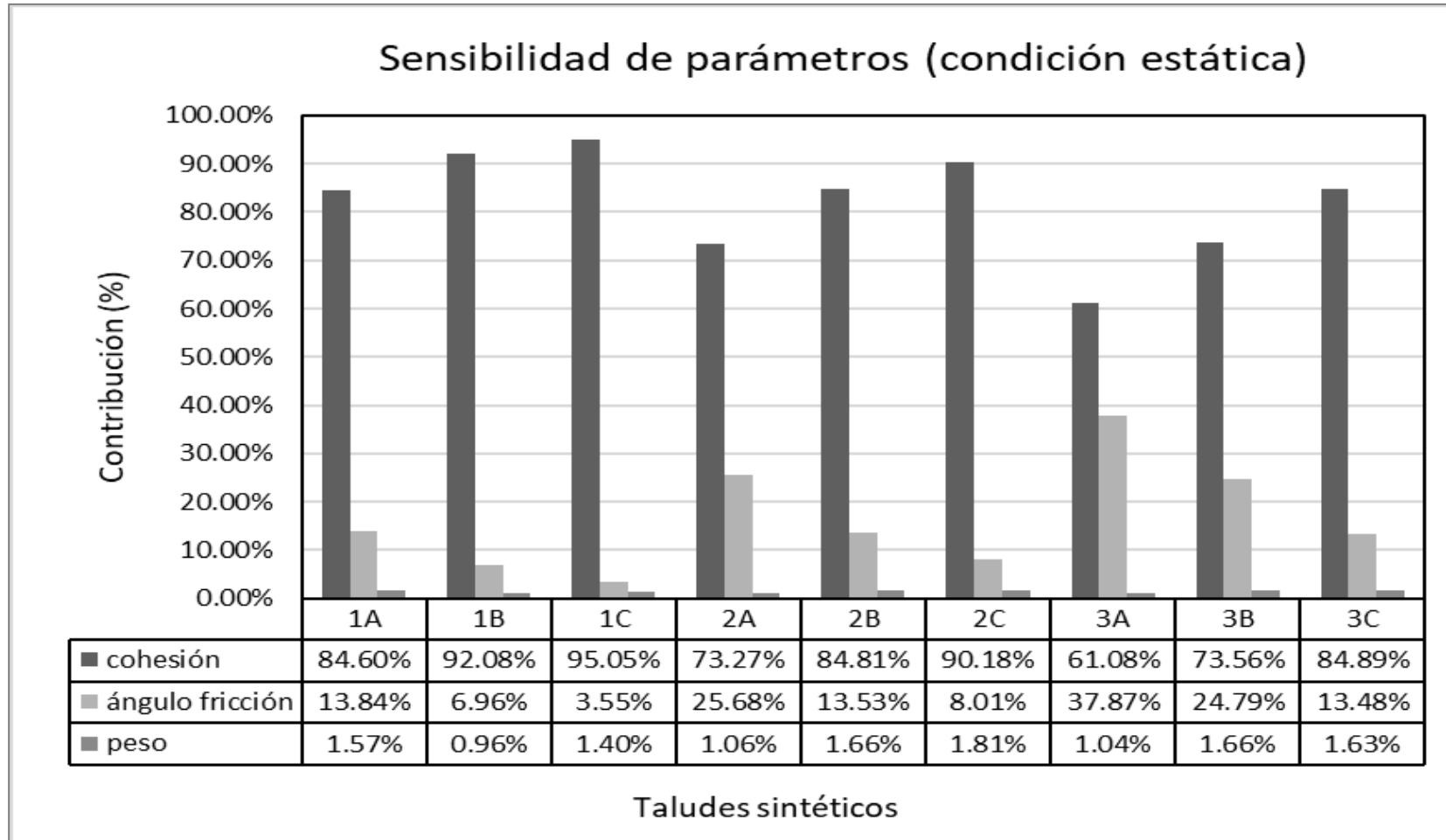
Results (1)

Casos	Incertidumbre	Condición estática				Condición pseudoestática			
		Spencer		M-P		Spencer		M-P	
		FSd	FSm	FSd	FSm	FSd	FSm	FSd	FSm
1A	CV _{mín}	2.364	2.365	2.363	2.365	1.480	1.481	1.483	1.480
	CV _{máx}	2.364	2.375	2.363	2.374	1.480	1.488	1.483	1.487
1B	CV _{mín}	1.608	1.604	1.602	1.602	1.151	1.152	1.145	1.151
	CV _{máx}	1.608	1.610	1.602	1.609	1.151	1.152	1.145	1.156
1C	CV _{mín}	1.189	1.126	1.192	1.107	<u>0.965</u>	1.051	<u>0.909</u>	<u>0.875</u>
	CV _{máx}	1.189	1.080	1.192	1.051	<u>0.965</u>	1.113	<u>0.909</u>	<u>0.739</u>
2A	CV _{mín}	1.894	1.893	1.892	1.894	1.178	1.178	1.179	1.181
	CV _{máx}	1.894	1.901	1.892	1.901	1.178	1.185	1.179	1.185
2B	CV _{mín}	1.214	1.211	1.210	1.210	<u>0.849</u>	<u>0.856</u>	<u>0.849</u>	<u>0.854</u>
	CV _{máx}	1.214	1.216	1.210	1.215	<u>0.849</u>	<u>0.857</u>	<u>0.849</u>	<u>0.858</u>
2C	CV _{mín}	<u>0.849</u>	<u>0.863</u>	<u>0.850</u>	<u>0.855</u>	<u>0.673</u>	<u>0.638</u>	<u>0.629</u>	<u>0.599</u>
	CV _{máx}	<u>0.849</u>	<u>0.847</u>	<u>0.850</u>	<u>0.853</u>	<u>0.673</u>	<u>0.620</u>	<u>0.629</u>	<u>0.581</u>

Results (2)

Casos	Incertidumbre	Condición pseudoestática							
		Spencer				M-P			
		FSd	FSm	Pf (%)	RI-n	FSd	FSm	Pf (%)	RI-n
1A	CV _{mín}	1.480	1.481	0.000	4.009	1.483	1.480	0.000	3.990
	CV _{máx}	1.480	1.488	1.120	2.184	1.483	1.487	1.190	2.178
1B	CV _{mín}	1.151	1.152	7.720	1.420	1.145	1.151	8.740	1.356
	CV _{máx}	1.151	1.152	20.97	0.803	1.145	1.156	21.49	0.780
1C	CV _{mín}	<u>0.965</u>	1.051	32.61	0.561	<u>0.909</u>	<u>0.875</u>	98.05	-1.23
	CV _{máx}	<u>0.965</u>	1.113	26.12	0.666	<u>0.909</u>	<u>0.739</u>	88.06	-1.03
2A	CV _{mín}	1.178	1.178	0.720	2.350	1.179	1.181	0.700	2.386
	CV _{máx}	1.178	1.185	11.78	1.204	1.179	1.185	11.70	1.206
2B	CV _{mín}	<u>0.849</u>	<u>0.856</u>	98.06	-2.11	<u>0.849</u>	<u>0.854</u>	97.64	-2.04
	CV _{máx}	<u>0.849</u>	<u>0.857</u>	86.60	-1.11	<u>0.849</u>	<u>0.858</u>	85.87	-1.07
2C	CV _{mín}	<u>0.673</u>	<u>0.638</u>	100.0	-9.28	<u>0.629</u>	<u>0.599</u>	100.0	-9.12
	CV _{máx}	<u>0.673</u>	<u>0.620</u>	100.0	-4.56	<u>0.629</u>	<u>0.581</u>	100.0	-4.78

Results (3)



Discussions, Conclusions & Recommendations (1)

- No significant differences are observed between the deterministic and probabilistic FS. It is impossible to detect the effect of uncertainty if the information provided by the probabilistic approach is ignored.
- The results of the slope analysis allowed us to identify cases where the stability criteria are verified, but not those of reliability. That is to say, although in these cases the slope is stable, it is not reliable.

Discussions, Conclusions & Recommendations (2)

- The parameters with the greatest influence on stability are cohesion, friction angle and soil weight, in that order of importance, although this may be altered in scenarios of maximum uncertainty and/or seismic activity.
- Reliability analyses in slope assessment do not guarantee that the designs proposed based on the results will never fail, but they do provide a broader overview for future decision-making.