

Study the Effective Location of Shear Wall and Its Cost of Multi-Story Building Under Seismic Loading in Iraq

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Abstract

The present research aims to study the response of multi-story framed building subjected to seismic loading. The finite element dynamic analysis is applied through using **SAP2000** software and using nonlinear static analysis in the study. The study includes calculating the free vibration analysis and the forced vibration due to earthquakes loading. The main factors considered in this study are the location of the shear wall and the type and economy cost of these types. The record of earthquake in Iraq is applied on a three dimensions multistory building with practical size and the response is determined and discussed. In our study will search the effective of the location and type of shear wall in the response of the building under earthquake loading and also we discuss the economy state for each case and compare between it to find the perfect case have good results under seismic loading and also have good economy cost. In our study, search the free vibration analysis and also can make forced vibration analysis under seismic loading. The study also uses multistory building above 15 story and the shear wall install along the building length and different location in center of the building, external corner, internal corner, and each parallel side in external and internal the building and also using different shape of share wall like angle and rectangular.

Keywords

Seismic Load, Shear Wall, Cost Management, Nonlinear Static Analysis, SAP2000

1. Introduction

Shear wall can be defined as any structural element that gives stiffness, strength and stability and is designed as a framed structure. Concrete walls such as shear walls and lift wells are normally used in multi-story buildings. Most people prefer it because of its ability to withstand strong winds and earthquakes. The structure is ideal by using mass center and coinciding centroid. A shear wall is normally required to stiffen the building. Thus, the function of shear wall is to increase the rigidity of structural system. Shear walls are normally used to construct modern building and make them appear vertical in nature. Lateral forces caused by blowing wind can make a building snap and bend over time. Therefore, there is need to use a shear wall to prevent such damages from taking place. Shear wall is used to provide earthquake resistance to concrete buildings. Building built using shear wall do not collapse easily even if there is an earthquake. In addition, the walls are used to separate enclosed close spaces. The shear wall can be 150 mm thick or 400mm high. The way the structure behaves depends on stiffness, weight

distribution; and strength of both the vertical and horizontal plane in a building. The shear walls are used in reducing earthquake effects. Furthermore, shear walls can also be used to improve a building's seismic response.

The seismic response is used to minimize the extent of earthquake on tall buildings. Shear walls are normally used in tall buildings to prevent them from collapsing. If done properly, shear walls can be useful in lateral force resisting. It is important to carry out an investigation on the appropriate location for placing the shear wall. Shear walls has gained a lot of popularity in construction of high buildings such as commercial towers. Therefore, shear walls has been found to be economical and effective for multi-story building. It is suitable for multi-story buildings of about 30 to 35 stories. Evidence shows that for the last 30 years no building constructed using shear walls has ever collapsed even when there are earthquakes and strong winds (Fintel, 1995). Thus, shear walls are preferred by many for constructing of buildings. For this study, different types of structures were used. There are structures without a shear wall and those that had a shear wall. The criteria that is used in RCC designing in seismic zones is lateral displacement from lateral forces. Investigations have been carried out to determine the most convenient location that can be used for lateral displacement of shear wall and also the base shear RCC frames. The location is an important consideration when it comes to constructing shear walls. The location will determine how long the building will remain standing without being damaged. Nonlinear statistical analysis also known as the pushover analysis was used to carry out the analysis on different kinds of models. Base shear and displacement are calculated from curves and then the two are compared against each other. Nonlinear analysis is an important tool in studying concrete behavior. The analysis helps in identifying the different characteristics of the concrete. Therefore, nonlinear analysis is a suitable method for giving an analysis on concrete behavior. Furthermore, it gives results that are easy to interpret.

2. Objective of the study

The main objective of the study is to find out how effective shear wall will be if it is placed at the right location.

3. Reinforced concrete shear walls

The reinforced concrete are normally vertical plate like walls. The reinforced concrete structure should be designed in such a way that they are able to carry combined loads like seismic loads, live and dead loads at a given level of safety. It should not be difficult to use reinforced concrete in such any kind of load. Therefore, it is vital to have an account of material properties, loads, method of analysis and structural system. Such information helps in identifying the amount of concrete that will be used for the construction. Additionally, having an account of material's convenient becomes it makes it a lot easier to determine how much material will be required. The walls are continuous for the entire part of the building. Breaking the walls will make it easy for the walls to become damaged in a short time. As such, there is need to take a lot of precaution before starting the construction work. The thickness of the walls are 150mm low and 400mm high in tall buildings. Shear walls play an important role in resisting earthquakes. It is for this reason that they have become popular in construction projects. According to one of the consulting engineers in the United States, Fintel (1995), there is no way concrete walls can be built without the use of shear walls. The statement shows just how important shear walls have become in almost every construction project.

Reinforced concrete shear walls give buildings stiffness and strength depending on their orientation direction. Therefore, RC walls help in reducing lateral sway and thus reducing the damage to the building and its contents. Shear walls have a large overturning effect because they support large earthquake forces. In order to reduce the effects of twist in structures, shear walls have to be symmetrical. It is the only way to avoid having massive losses. The walls can be placed in any direction but they have to be symmetrically placed. For shear walls to be effective, they have to be located along an exterior perimeter. There is need to stress on the location of the shear wall since it determines whether the building will

withstand earthquakes or not. Making use of the exterior perimeter helps reduce the chances of the building succumbing to twisting. Therefore, no construction work should lack the exterior perimeter.

4. Shear walls function

There are numerous benefits that shear walls offer to building. First of all, shear walls have to provide the strength needed to resist earthquakes. If the shear walls are stronger then it will be a lot easier for them to transfer their horizontal forces to the element that follows. The next element may be a floor, shear wall, foundation walls, footings or slabs. It does not matter the type of element. What matters is whether the shear wall is able to transfer the horizontal forces. In case the shear wall does not transfer the forces, then it increases the chances of the building being damaged. Apart from that, shear walls give lateral stiffness required to prevent floor or roof from side sway. Additionally, stiff shear walls help prevent roof and floor from shifting away from their supports. Furthermore, stiff buildings do not have nonstructural damage. The function of shear walls in any building cannot be stressed enough.

4.1 Analysis

The analysis of the building is done by using the SAP 2000 software. The software is used to run all the analysis. The software can predict the nonlinear behavior under dynamic and static loadings while taking into account the material inelasticity and geometric nonlinearity. The SAP2000 works well with both dynamic and static loads. It can also perform nonlinear dynamic analysis and nonlinear static pushover. The software is a convenient means for conducting the analysis on shear walls. The models used were designed in the SAP2000 software using different section of the reinforced shear wall, cross type, L type, box type shear wall. The sections are located in different areas like at corner, along periphery, and middle positions. They locations are separated in that manner to determine the appropriate location for placing the shear wall.

The buildings used are modeled to resist load elements. Therefore, there is no need to worry about exceeding the load elements. The loads to be applied on the structures are from Iraq standards. The standards are from Iraq since that is where the study will take place. The buildings used in the study have both brick masonry and reinforced concrete elements. This is to help determine the most convenient concrete for use with shear walls. It is assumed that the frames are fixed firmly at the bottom of the structure. Therefore, they will not interrupt in any way with the construction of the shear walls. The construction process will therefore be done without any interruption.

4.2 Nonlinear static analysis

The pushover analysis is a nonlinear procedure whereby lateral loads magnitudes is increased thus giving a predefined distribution on the height of the structure. Pushover analysis can be used to determine how a building behaves.

4.3 Purpose pushover analysis

The pushover analysis is normally used to give an evaluation of expected performance of buildings. The performance is evaluated by analyzing the deformation and strength demands. The pushover analysis is normally carried out for all models. Therefore, it is suitable for use in this study.

4.4 Problem statement

To carry out the analysis, the model of a reinforced building and the plan area that has been selected is in Iraq. The height of the story building is about 3.5 m while the height of floor-to-floor is three. The spacing

frame to be used is 4m. The concrete used for the analysis is M20 while the structural steel is Fe415. The structural properties to be used for the reinforced concrete shear as are; shear wall thickness of 200mm, total depth of the slab will be 120mm, the external thickness of the wall is 250mm, the internal thickness of the wall is 150mm, the external column size is 300 by 530mm, the zone factor is 0.1, the importance factor is 1, and the response reduction factor is 3.

4.5 Methodology

This section looks at the methodology of the study. It gives a clear explanation of the methodology used to conduct the study. There are assumptions used in conducting the study. The building has 20 stories. The columns are assumed to be fixed. The slab is 0.15m for all the stores; the floor-to-floor height is 3.5m. The live load on roof is 4 kN/m^2 , the floor finish is 1 kN/m^2 , the live load on the roof is 1.5 kN/m^2 . The wall thickness for all the beams is set at 230mm. The seismic weight is 24 kN/m^2 . The concrete grade used is M-30 for the slab and M-40 for the beam.

The models that were used in the analysis were;

- Buildings with shear walls and uniform thickness
- Building with shear walls and varying thickness

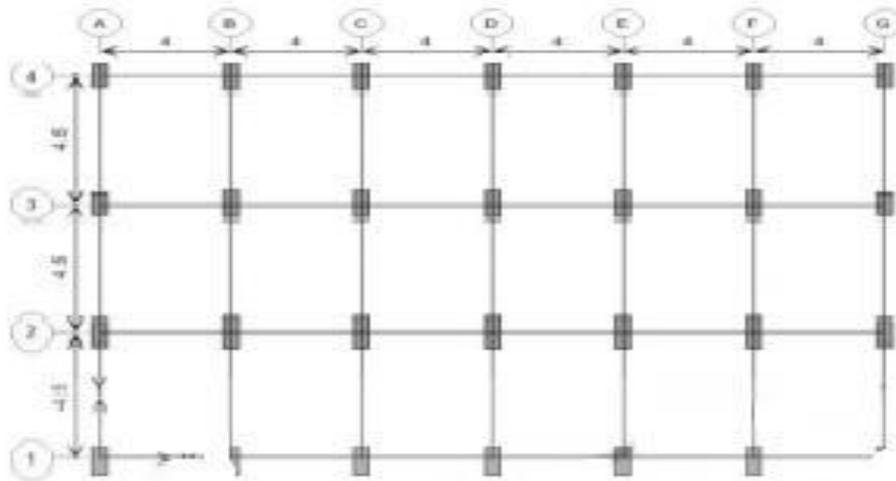


Figure 1: Plan design of 20-story building

5. Result and discussion

The analysis as carried out to determine how concrete structure perform under seismic loading. The results in form of story drift and story displacement and the result variation has been discussed. The method used to carry out the building analysis is nonlinear static method. Furthermore, the variation of the story drift and story displacement for building with shear walls with varying and uniform thickness has also been discussed. The results are as follows:

5.1 Displacement

As it is evident from the figures below, the displacement of the story, reduce when a T2-B model is developed. The maximum displacement is on the T2-A model while the minimum displacement is T2-B model. The reduction in displacement of the stories is as a result of the increase in structure stiffness and even on the velocity, the displacement is lowest on the stories

at the bottom and is high on the upper stories. The displacement is with regards to the structural strength, stability and human comfort. Given that the displacement for T2B is less as compared to the other model; its means that the T2B is more stable.

Table 1: Story displacement of type 1 model

| STOREY | T1AUX | T1BUX | T1CUX | T1DUX |
|------------|----------|----------|----------|----------|
| Story 1-20 | 691.2504 | 589.1949 | 541.6675 | 663.5825 |
| Story 1-19 | 649.398 | 553.0885 | 506.8151 | 624.6544 |
| Story 1-18 | 607.0139 | 516.5876 | 471.8105 | 585.0707 |
| Story 1-17 | 564.1682 | 479.7782 | 436.7386 | 544.9033 |
| Story 1-16 | 520.971 | 442.7754 | 401.71 | 504.2648 |
| Story 1-15 | 477.5823 | 405.7243 | 366.8603 | 463.3127 |
| Story 1-14 | 434.2087 | 368.7959 | 332.3418 | 422.2469 |
| Story 1-13 | 391.087 | 332.1899 | 298.3203 | 381.2979 |
| Story 1-12 | 348.4865 | 296.1197 | 264.9806 | 340.7259 |
| Story 1-11 | 306.7082 | 260.8087 | 232.5229 | 300.8083 |
| Story 1-10 | 266.0676 | 226.5011 | 201.1604 | 261.8379 |
| Story 1-9 | 226.8931 | 193.4565 | 171.1196 | 224.1314 |
| Story 1-8 | 189.5373 | 161.9496 | 142.6381 | 188.0247 |
| Story 1-7 | 154.3711 | 132.2696 | 115.9639 | 153.8726 |
| Story 1-6 | 121.7812 | 104.719 | 91.354 | 122.0484 |
| Story 1-5 | 92.1664 | 79.6143 | 69.0741 | 92.945 |
| Story 1-4 | 65.9342 | 57.2865 | 49.397 | 66.9729 |
| Story 1-3 | 43.4972 | 38.0812 | 32.603 | 44.5614 |
| Story 1-2 | 25.2695 | 22.3616 | 18.9793 | 26.162 |
| Story 1-1 | 11.6642 | 10.5082 | 8.8154 | 12.2516 |
| Story 1 | 3.1041 | 2.9181 | 2.3994 | 3.3423 |

Table 2: Story wise displacement of type 2 model

| STOREY | T2AUX | T2BUX | T2CUX | T2DUX |
|------------|----------|----------|----------|----------|
| Story 1-20 | 723.8406 | 520.8793 | 587.7572 | 619.1219 |
| Story 1-19 | 679.4758 | 486.3515 | 549.5401 | 581.8812 |
| Story 1-18 | 634.186 | 451.5279 | 510.9222 | 543.6996 |
| Story 1-17 | 588.1431 | 416.5681 | 472.1014 | 504.7251 |
| Story 1-16 | 541.7327 | 381.7457 | 433.4237 | 465.3003 |
| Story 1-15 | 495.1849 | 347.1562 | 394.9524 | 425.6303 |
| Story 1-14 | 448.781 | 313.0081 | 356.8988 | 385.9824 |
| Story 1-13 | 402.851 | 279.5312 | 319.4987 | 346.6611 |
| Story 1-12 | 357.7742 | 246.9694 | 283.02 | 307.9947 |
| Story 1-11 | 313.9539 | 215.5895 | 247.7467 | 270.3259 |
| Story 1-10 | 271.7042 | 185.5945 | 213.9412 | 233.9564 |
| Story 1-9 | 231.145 | 157.0162 | 181.6168 | 199.0065 |
| Story 1-8 | 192.6465 | 130.1062 | 151.0603 | 165.813 |
| Story 1-7 | 156.6125 | 105.1305 | 122.5711 | 134.7365 |
| Story 1-6 | 123.4733 | 82.367 | 96.4686 | 106.1597 |
| Story 1-5 | 93.6865 | 62.098 | 73.0839 | 80.4846 |
| Story 1-4 | 67.4694 | 44.4496 | 52.5898 | 57.9043 |
| Story 1-3 | 44.8741 | 44.8741 | 34.9671 | 38.4621 |
| Story 1-2 | 26.337 | 26.337 | 20.5442 | 22.5353 |
| Story 1-1 | 12.3276 | 12.3276 | 9.6678 | 10.5256 |
| Story 1 | 3.3589 | 3.3589 | 2.6949 | 2.8603 |

5.2 Story drift ratio

The story drift refers to the displacement of one level compared to another that is above or below it. Stories may exhibit different kinds of displacement. Therefore, when the story structure decreases, the probability of the building collapsing is also reduced. The model type two will help in reducing the structure response. The drift ratios for type two B model are less as compare to those of type 2A model. The story drift ratio is reduced in the bottom stories, the ratios become high in middle stories and decreases in the upper stories.

Table 3: Story drift for model type 1

| STOREY | T2AUX | T2BUX | T2CUX | T2DUX |
|------------|----------|----------|----------|----------|
| Story 1-20 | 0.011039 | 0.010316 | 0.010778 | 0.011317 |
| Story 1-19 | 0.011138 | 0.010429 | 0.010874 | 0.011517 |
| Story 1-18 | 0.011214 | 0.010517 | 0.010946 | 0.011695 |
| Story 1-17 | 0.011258 | 0.010572 | 0.010983 | 0.011841 |
| Story 1-16 | 0.011264 | 0.010586 | 0.010979 | 0.011942 |
| Story 1-15 | 0.011226 | 0.010551 | 0.010927 | 0.011986 |
| Story 1-14 | 0.011137 | 0.010459 | 0.010819 | 0.011962 |
| Story 1-13 | 0.010992 | 0.010306 | 0.01065 | 0.01186 |
| Story 1-12 | 0.010788 | 0.010089 | 0.010416 | 0.011676 |
| Story 1-11 | 0.010525 | 0.009802 | 0.01011 | 0.011407 |
| Story 1-10 | 0.010194 | 0.009441 | 0.009729 | 0.011045 |
| Story 1-9 | 0.009783 | 0.009002 | 0.009268 | 0.010586 |
| Story 1-8 | 0.009268 | 0.00848 | 0.008725 | 0.010022 |
| Story 1-7 | 0.008645 | 0.007872 | 0.008093 | 0.009348 |
| Story 1-6 | 0.007909 | 0.007173 | 0.00737 | 0.008558 |
| Story 1-5 | 0.007057 | 0.006379 | 0.006552 | 0.007647 |
| Story 1-4 | 0.006086 | 0.005487 | 0.005633 | 0.006609 |
| Story 1-3 | 0.004993 | 0.004491 | 0.004609 | 0.005435 |
| Story 1-2 | 0.003781 | 0.003387 | 0.003474 | 0.004118 |
| Story 1-1 | 0.002435 | 0.002169 | 0.002224 | 0.002645 |
| Story 1 | 0.001021 | 0.000834 | 0.000856 | 0.000998 |

Table 4: Story drift for model type 2

| STOREY | T2AUX | T2BUX | T2CUX | T2DUX |
|------------|----------|----------|----------|----------|
| Story 1-20 | 0.012676 | 0.010395 | 0.010919 | 0.011132 |
| Story 1-19 | 0.01294 | 0.010517 | 0.011034 | 0.011444 |
| Story 1-18 | 0.013155 | 0.010592 | 0.011092 | 0.011712 |
| Story 1-17 | 0.01326 | 0.010578 | 0.011051 | 0.011871 |
| Story 1-16 | 0.013299 | 0.010534 | 0.011 | 0.011968 |
| Story 1-15 | 0.013258 | 0.010427 | 0.010902 | 0.011986 |
| Story 1-14 | 0.013123 | 0.01025 | 0.010733 | 0.011913 |
| Story 1-13 | 0.012879 | 0.009998 | 0.010483 | 0.011741 |
| Story 1-12 | 0.01252 | 0.009665 | 0.010149 | 0.011466 |
| Story 1-11 | 0.012071 | 0.009266 | 0.009736 | 0.011094 |
| Story 1-10 | 0.011588 | 0.008858 | 0.009317 | 0.010686 |
| Story 1-9 | 0.011 | 0.00837 | 0.008814 | 0.010175 |
| Story 1-8 | 0.010295 | 0.007798 | 0.008223 | 0.009554 |
| Story 1-7 | 0.009468 | 0.007138 | 0.007539 | 0.008815 |
| Story 1-6 | 0.008511 | 0.006386 | 0.006759 | 0.007951 |
| Story 1-5 | 0.007491 | 0.005588 | 0.005927 | 0.00702 |
| Story 1-4 | 0.006456 | 0.004795 | 0.0051 | 0.00607 |

| | | | | |
|-----------|----------|----------|----------|----------|
| Story 1-3 | 0.005297 | 0.003919 | 0.004177 | 0.004998 |
| Story 1-2 | 0.004005 | 0.002947 | 0.003153 | 0.003793 |
| Story 1-1 | 0.002565 | 0.001883 | 0.002023 | 0.002442 |
| Story 1 | 0.000961 | 0.000722 | 0.000784 | 0.000925 |

5.3 Maximum deflection

The deflection in the shear wall model along the periphery is reduced. The column reduces to 33.33% as compared to those model constructed in the L shape shear wall.

5.4 Maximum force in the beams

The earthquake effects at ground story is higher as compared to the middle and top levels. For a given beam the ground storey increases by about 21.21%.

6. Conclusion

The study carried out to show the earthquake response in tall buildings. Different kinds of shear thickness and position have been used. The purpose for the study was to investigate on the whether the location of the shear wall affects its seismic loading. The conclusion that was reached is as follows;

Providing the shear walls at the right location reduce the amount of displacement caused by the earthquakes. Additionally, the percentage of the drift depends on the shear wall thickness location. As such, it is easier to determine the drift ratio.

The model that had shear wall and varying thickness had the highest displacement in the elastic region. Therefore, such a building acts well on the elastic region. There will be no need of looking for a suitable place to construct it than the elastic region.

Upon observation, it was evident that the story drift ratios of the type 2 model were less as compared to the ratios of the type 1 model. The story drift ratio in the bottom storey is very low and it increases in the middle and finally reduces at the upper stories.

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